Ministry of Urban Development Department of Urban Development & Building Construction Building Code and By - Laws Section Babarmahal, Kathmandu

NBC Compliance Check List on Structural Analysis & Design for RCC

S.No.	General	Informtion:		
1	Owner's Name:			
2	Address:			
3	Location of Building			
4	Occupancy Type of the Building as per Byelaws:			
5	Name of the Structural Designer:			
6	Nepal Engineeering Council No.:			
7	Contact Number of the Structural Designer:			
8	E-Mail ID of Designer:			
9	Name of the Consulting Firm:			
S. No.	Description	Input Data	Units	Remarks
A	Geometrical Configuration of Building:			
A.1	No. of Block		no.	
A.2	Number of Storeys Considered in the Design		no.	
A.3	Design Provision for Future Extension:	[]Yes []No		
	(Future Extension No. of Additional Floors) If Yes			
A.4	Plan Shape of Individual Block			
A.5	Story Height:			
	i. Lower Basement		m	
	ii. Upper basement		m	
	iii. Ground Floor		m	
	iv. Typical Floor		m	
A.6	Total Height of Building Structure		m	
A.7	Height Considered of Fundamntal Time Period Calculation		m	
A.8	Length of Building (L)		m	
A.9	Width of Building (B)		m	
A.10	Height to Width Ratio of Building (H/W)			
A.11	Length to Width Ratio of Building (L/W)			
A.12	Parapet Height		m	
A.13	Wall Type (External & Internal)			
	i. External Wall + Plaster Thickness:		mm	
	ii. Internal Wall + Plaster Thickness:		mm	
	iii. Others			
	External & Internal Wall Partition	[]Brick []AAC []Boar		
	No. of Column (Plinth Area):		no.	
A.16	No. of Lift:		no.	
A.17	Type of Lift Casing Material According to Construction Materials			
A.18	No. of Staircase:		no.	
A.19	Type of Staircase According to Construction Materials			
A.20	Roof System:			
В	Defining Basic Material Properties of the Building.			
B.1	Material Properties			
	a) Characteristics Strength of Concrete Grade:			
	i. Columns		MPa	
	ii. Beams		MPa	
	iii. Slabs		MPa	Clause 2.1
	iv. Shear Wall (If Applicable)		MPa	Annex : A
	v. Foundations (If Applicable)		MPa	
	vi. Pile Foundations (If Applicable)		MPa	

	i. Flexural Reinforcement			MPa	Clause 2.1 (A)
	ii. Confinement Reinforcement (Ties)			MPa	
D A					1
B.2	Density of Materials:				NBC 101:199
	i. Weight Density of Concrete			kN/m3	
B.3	Mechanical Properties of Construction Materials:				
	i. Masonry Weight			kN/m3	NBC 101:199
	ii. Rebar Weight per Unit			kN/m3	
B.4	iii. Other			kN/m3	
	Load Patterns Considered				1
D.4	a) Dead Loads				
	Self-weight, Wall Load, Floor Finishing, Partition Load, Parapet Walls,	[]Yes	[] No		NBC 102:199
	Staircase Dead Load, Machinary Dead Load, Water Tank Load.				4
	Other b) Imposed Loads				
	Floor Live Loads (Non-Storage Type and Storage Type), Roof Live Load,			1	-
	Staircase Live Load.	[]Yes	[] No		NBC 103:199
	Others				
		Г Т Т Т	F 137	1	
	c) Wind Load	[]Yes	[] No		NBC 104:199
	If No Note: For this Building, The Seismi Load was found larger than Wind	Load Hence The W	ind Load was not Acc	ounted for Design of th	ne Building
	Tote. For this Dunning, The Seisin Loun was jound unger than white	Louis menee, the W	Loui was not Acc	omnen jor Design of th	Эшиннъ.
B.5	Detailed Load Calculations				NBC 101:199
	i. Detailed Load Calculations	[] Yes	[] No		NBC 101:199
D (D'andre 8 Easter 1.14				1
B.6	Direction & Eccentricity i. X Direction:	[]Yes	[] No	1	-
	ii. X Direction + Eccentricity:	[]] Yes	[]No		- Clause 5.7
	iii. X Direction - Eccentricity:	[] Yes	[]No		-
	III I Bheenen Levenatory.				
				1	
B.7	Accidental Eccentricity	X-direction	Y-direction		Clause 5.7
B.7	Accidental Eccentricity i. Acc. Eccentricity Considered in X and Y-Direction	X-direction	Y-direction	%	- Clause 5.7
	i. Acc. Eccentricity Considered in X and Y-Direction	X-direction	Y-direction	%	- Clause 5.7
B.7 B.8	i. Acc. Eccentricity Considered in X and Y-Direction Story Range	X-direction	Y-direction	%	
	i. Acc. Eccentricity Considered in X and Y-Direction Story Range i. Top Story :	X-direction	Y-direction	%	
	i. Acc. Eccentricity Considered in X and Y-Direction Story Range i. Top Story : ii. Bottom Story :	X-direction	Y-direction	%	
	i. Acc. Eccentricity Considered in X and Y-Direction Story Range i. Top Story : ii. Bottom Story : Load Combinations:	X-direction	Y-direction	%	
B.8	i. Acc. Eccentricity Considered in X and Y-Direction Story Range i. Top Story : ii. Bottom Story : Load Combinations: a) For Parallel System			%	From Softwar
B.8	i. Acc. Eccentricity Considered in X and Y-Direction Story Range i. Top Story : ii. Bottom Story : Load Combinations: a) For Parallel System i. 1.2*DL+1.5*LL	[] Yes	[] No	%	From Softwar
B.8	i. Acc. Eccentricity Considered in X and Y-Direction Story Range i. Top Story : ii. Bottom Story : Load Combinations: a) For Parallel System i. 1.2*DL+1.5*LL ii. DL + λLL ± EX	[] Yes [] Yes	[] No [] No	%	From Softwar
B.8	i. Acc. Eccentricity Considered in X and Y-Direction Story Range i. Top Story : ii. Bottom Story : Load Combinations: a) For Parallel System i. 1.2*DL+1.5*LL	[] Yes	[] No	%	From Softwar
B.8	i. Acc. Eccentricity Considered in X and Y-Direction Story Range i. Top Story : ii. Bottom Story : Load Combinations: a) For Parallel System i. 1.2*DL+1.5*LL ii. DL + λLL ± EX	[] Yes [] Yes	[] No [] No	%	From Softwar
B.8	i. Acc. Eccentricity Considered in X and Y-Direction Story Range i. Top Story : ii. Bottom Story : Load Combinations: a) For Parallel System i. 1.2*DL+1.5*LL ii. DL + λLL ± EX iii. DL + λLL ± EY	[] Yes [] Yes	[] No [] No		From Softwar Clause 3.6 Clause 3.6.1
B.8	i. Acc. Eccentricity Considered in X and Y-Direction Story Range i. Top Story : ii. Bottom Story : Load Combinations: a) For Parallel System i. $1.2*DL+1.5*LL$ ii. DL + $\lambda LL \pm EX$ iii. DL + $\lambda LL \pm EX$ iii. DL + $\lambda LL \pm EY$ b) For Non-Parallel System	[] Yes [] Yes [] Yes	[] No [] No [] No [] No		From Softwar Clause 3.6 Clause 3.6.1
B.8	i. Acc. Eccentricity Considered in X and Y-Direction Story Range i. Top Story : ii. Bottom Story : Load Combinations: a) For Parallel System i. $1.2*DL+1.5*LL$ ii. DL + $\lambda LL \pm EX$ iii. DL + $\lambda LL \pm EX$	[] Yes [] Yes [] Yes [] Yes	[] No [] No [] No [] No		From Softwar Clause 3.6 Clause 3.6.1
B.8	i. Acc. Eccentricity Considered in X and Y-Direction Story Range i. Top Story : ii. Bottom Story : Load Combinations: a) For Parallel System i. $1.2*DL+1.5*LL$ ii. DL + $\lambda LL \pm EX$ iii. DL + $\lambda LL \pm EX$ iii. DL + $\lambda LL \pm EY$ b) For Non-Parallel System i. $1.2*DL+1.5*LL$ ii. DL+ $\lambda LL \pm (EX \pm 0.3*EY)$ iii. DL+ $\lambda LL \pm (EY \pm 0.3*EX)$	[] Yes [] Yes [] Yes [] Yes [] Yes	[] No [] No [] No [] No [] No [] No		From Softwar Clause 3.6 Clause 3.6.1
B.8	i. Acc. Eccentricity Considered in X and Y-Direction Story Range i. Top Story : ii. Bottom Story : Load Combinations: a) For Parallel System i. 1.2*DL+1.5*LL ii. DL + λ LL ± EX iii. DL + λ LL ± EX iii. DL + λ LL ± EY b) For Non-Parallel System i. 1.2*DL+1.5*LL ii. DL+ λ LL ± (EX ± 0.3*EY) iii. DL+ λ LL ± (EY ± 0.3*EX) 	[] Yes [] Yes [] Yes [] Yes [] Yes	[] No [] No [] No [] No [] No [] No		From Softwar Clause 3.6 Clause 3.6.1
B.8	i. Acc. Eccentricity Considered in X and Y-Direction Story Range i. Top Story : ii. Bottom Story : Load Combinations: a) For Parallel System i. 1.2*DL+1.5*LL ii. DL + λ LL ± EX iii. DL + λ LL ± EX iii. DL + λ LL ± EY b) For Non-Parallel System i. 1.2*DL+1.5*LL ii. DL+ λ LL ± (EX ± 0.3*EY) iii. DL+ λ LL ± (EY ± 0.3*EX) 	[] Yes [] Yes [] Yes [] Yes [] Yes	[] No [] No [] No [] No [] No [] No		From Softwar Clause 3.6 Clause 3.6.1
B.8	i. Acc. Eccentricity Considered in X and Y-Direction Story Range i. Top Story : ii. Bottom Story : Load Combinations: a) For Parallel System i. 1.2*DL+1.5*LL ii. DL + λ LL ± EX iii. DL + λ LL ± EX iii. DL + λ LL ± EY b) For Non-Parallel System i. 1.2*DL+1.5*LL ii. DL+ λ LL ± (EX ± 0.3*EY) iii. DL+ λ LL ± (EY ± 0.3*EX) 	[] Yes [] Yes [] Yes [] Yes [] Yes	[] No [] No [] No [] No [] No [] No		From Softwar Clause 3.6 Clause 3.6.1
B.8	i. Acc. Eccentricity Considered in X and Y-Direction Story Range i. Top Story : ii. Bottom Story : Load Combinations: a) For Parallel System i. $1.2*DL+1.5*LL$ ii. $DL + \lambda LL \pm EX$ iii. $DL + \lambda LL \pm EX$ iii. $DL + \lambda LL \pm EY$ b) For Non-Parallel System i. $1.2*DL+1.5*LL$ ii. $DL + \lambda LL \pm (EX \pm 0.3*EY)$ iii. $DL + \lambda LL \pm (EY \pm 0.3*EX)$ c) Others i. ii.	[] Yes [] Yes [] Yes [] Yes [] Yes	[] No [] No [] No [] No [] No [] No		From Softwar Clause 3.6 Clause 3.6.1
B.8	i. Acc. Eccentricity Considered in X and Y-Direction Story Range i. Top Story : ii. Bottom Story : Load Combinations: a) For Parallel System i. 1.2*DL+1.5*LL ii. DL + λ LL ± EX iii. DL + λ LL ± EY b) For Non-Parallel System i. 1.2*DL+1.5*LL ii. DL+ λ LL ± (EX ± 0.3*EY) iii. DL+ λ LL ± (EY ± 0.3*EX) C) Others i. ii. ii. ii. Mass Source Considered for Seismic Weight	[] Yes [] Yes [] Yes [] Yes [] Yes [] Yes	[] No [] No [] No [] No [] No [] No [] No		From Softwar Clause 3.6 Clause 3.6.1
B.8 B.9	i. Acc. Eccentricity Considered in X and Y-Direction Story Range i. Top Story : ii. Bottom Story : Load Combinations: a) For Parallel System i. 1.2*DL+1.5*LL ii. DL + λ LL ± EX iii. DL + λ LL ± EY b) For Non-Parallel System i. 1.2*DL+1.5*LL ii. DL+ λ LL ± (EX ± 0.3*EY) iii. DL+ λ LL ± (EY ± 0.3*EX) C) Others i. ii. ii. ii. ii. ii. ii. ii.	[] Yes [] Yes [] Yes [] Yes [] Yes [] Yes [] Yes	[] No [] No [] No [] No [] No [] No [] No		From Softwar Clause 3.6 Clause 3.6.1 Clause 3.6.2 Clause 5.2
B.8 B.9	i. Acc. Eccentricity Considered in X and Y-Direction Story Range i. Top Story : ii. Bottom Story : Load Combinations: a) For Parallel System i. 1.2*DL+1.5*LL ii. DL + λ LL ± EX iii. DL + λ LL ± EY b) For Non-Parallel System i. 1.2*DL+1.5*LL ii. DL+ λ LL ± (EX ± 0.3*EY) iii. DL+ λ LL ± (EY ± 0.3*EX) C) Others i. ii. ii. ii. Mass Source Considered for Seismic Weight	[] Yes [] Yes [] Yes [] Yes [] Yes [] Yes	[] No [] No [] No [] No [] No [] No [] No		From Softwar Clause 3.6 Clause 3.6.1 Clause 3.6.2

	If Yes				1
B.12	Modal Damping:			%	Clause 1.3
		•			
C	Calculation of Seismic Load as per (NBC 105:2020)				
C.1	Structural System:		t Static Method (E	<i>,</i>	Clause 6
		[] Modal Re	sponse Spectrum N	Iethod (MRSM)	Clause 7
C.2	Seismic Zoning Factor, (Z)				Clause 4.1.4 / T. 4
C.3	Type of Building				
C.4	Importance Class (I, II, III)	[] I [] II [] III	Clause 4.1.5
C.5	Importance Factor, (I)				Table 4-6
C.6	Height of the Building, (H)			m	
C.7	Type of Structure		Resisting Concrete		_
			Resisting Steel Fran	me Systems	_
			rame Systems		Clause 5.4.2
			l Wall Systems		_
		[] Dual Syst	tems	1	
C.8	Factor, k _t =				Clause 5.1.2
C.9	Approximate Fundamental Time Period Empirical Equation, T ₁ =kt*H _{0.75}			sec	
C.10	Amplification of Approximate Time Period, T ₁ '=1.25*T1			sec	Clause 5.1.3
C.11	Fundamental Time Period from Rayleigh's Formula, T ₁			sec	Clause 5.1.1
C.12	Adopted Time Period, T ₁ (Refer C.10)			sec	Clause 5.1.2
					1
C.14	Spectral Shape Factor:			1	
	i. Site Sub-Soil Type	C	D		Clause 4.1.3
	ii. Spectral Shape Factor, C _h (T) for ESM	2.5	2.25		_
	iii. The Lower Period of the Flat Part of the Spectrum, T _a for ESM	0			_
	iv. The Lower Period of the Flat Part of the Spectrum, T _a for RSM	0.1	0.5		Clause 4.1.2
	v. The Upper Period of the Flat Part of the Spectrum, T _c	1	2		Table 4-1
	vi. Peak Spectral Acceleration Normalized by PGA, a	2.5	2.25		_
	vii. Coefficient that Controls the Descending Branch of the Spectrum, K	1.8	0.8		
C.15	Calculation of Elastic Site Spectra				
0.15	a) Elastic Site Spectra for Horizontal Loading for Ultimate Limit State (UL	S)			
	C(T) = Ch(T)*Z*I=				- Clause 4.1.1
	b) Elastic Site Spectra for Horizontal Loading for Serviceabilty Limit State	(SLS)			
	$C_s(T)=0.20*C(T)=$				Clause 4.2
	c) Elastic Site Spectra for Vertical Loading,				
	$\frac{C_{V}(T_{v})=(2/3)*C(T)=}{C_{v}(T_{v})=(2/3)*C(T)=}$	1		1	- Clause 4.3
	$C_{V}(\Gamma_{V})$ (2.5) $C(\Gamma)$				
C.16	Ductility & Overstrength Factors:				
	a) Ultimate Limit State (ULS):				Clause 5.3.1 / 5.4
	i. Ductilty Factor for ULS, R_{μ}				Table 5-2
	ii. Overstrength Factor for ULS, Ω_{μ}				Tuble 5-2
	k) Samiaaakilita Limit Stata (SLS).				Clause 5.3.2 / 5.4
	b) Serviceability Limit State (SLS): i. Ductilty Factor for SLS, R _s				Cuuse 5.5.27 5.4
	ii. Overstrength Factor for SLS, Ω_s				Table 5-2
	n. Overstrength Factor for 52.5, 52 _s				
C.17	Calculation of Horizontal Base Shear Coefficients:			Clause 6.1	
	a) For Ultimate Limit State (ULS), $C_d(T_1) = C(T1)/(R\mu^*\Omega\mu)$		(Seismic	Coefficient)	Clause 6.1.1
	b) Serviceability Limit State (SLS), $C_d(T_1) = C_s(T_1)/\Omega_s$		(Seismic	Coefficient)	Clause 6.1.2
C.18	Calculation of Horizontal Base Shear:				
	a) Seismic Weight, W			kN	(From ETABS)
	b) For Ultimate Limit State (ULS), V= Cd(T1)*W			kN	, , ,
	c) For Serviceabilty Limit State (SLS), V=Cd(T1)*W		1	kN	- Clause 6.2
~ • *				•	
C.19	Exponent for Vertical Distribution of Seismic Forces	T	K		Clause 6.3
	i. For Structure having Tie Period T \leq 0.5 sec, k=1	0.5	1	1	1

	ii. Exponent for Vertical Distribution of Seismic Forces, k			k	
	iii. For Structure having Tie Period T \leq 2.5 sec, k=2	2.5	2		
C.20	Initial Scale Factor for Scaling of Base Shear in MRSM	0010			
	Acceleration due to gravity (g) =	9810		m/sec	
	a) Ultimate Limit State:				
	i. Factor in X-direction, SFx= $Z^*I^*g/(R\mu x\Omega\mu)$				
	ii. Factor in Y-direction, SFy= $Z^*I^*g/(R\mu x\Omega\mu)$				Clause
	h) Samiasahilty Limit States				Cuuse
	b) Serviceabilty Limit State: i. Factor in X-Direction, SFx= 0.2*Z*I*g/(RμxΩμ)				
	ii. Factor in Y-Direction, SFy= $0.2*Z*I*g/(R\mu x 2\mu)$				
	$\frac{1}{2} \frac{1}{2} \frac{1}$				
D	Modeling of the Building				
D.1	Software used for design of structure along with version :				Version
D.2	Element Sizes:				
	i. Column Sizes				
		-			
	ii. Main Beam Sizes				
		-			
	iii. Secondary Beam Sizes				
		-			From Software
	iv. Slab Thickness				1.000.20070.000
		-1			
	v. Staircase Waist Slab Thickness				
		-			
	vi. Shear Wall Thickness (if applicable)				
		-			
D.3	Types of Slabs:	[] Slab []	Drop [] Ribb	ed []Waffle	From Software
		·			
D.4	Slab Modeling Type:	[] S-Thin []] S-Thick [] Me	mbr. [] Layered	From Software
		[] S-Thin []]S-Thick [] Me	mbr. [] Layered	From Software
D.4 D.5	Property/Stiffness Modifiers:			mbr. [] Layered	From Software
	Property/Stiffness Modifiers: a) Columns:	[]S-Thin[] Flexural Stiff.	S-Thick [] Me	mbr. [] Layered	
	Property/Stiffness Modifiers: a) Columns: i. Shear Area in 2 Direction			mbr. [] Layered	Clause 3.4
	Property/Stiffness Modifiers: a) Columns: i. Shear Area in 2 Direction ii. Shear Area in 3 Direction			mbr. [] Layered	
	Property/Stiffness Modifiers: a) Columns: i. Shear Area in 2 Direction ii. Shear Area in 3 Direction iii. Moment of Interia about 2 Axis			mbr. [] Layered	Clause 3.4
	Property/Stiffness Modifiers: a) Columns: i. Shear Area in 2 Direction ii. Shear Area in 3 Direction			mbr. [] Layered	Clause 3.4
	Property/Stiffness Modifiers: a) Columns: i. Shear Area in 2 Direction ii. Shear Area in 3 Direction iii. Moment of Interia about 2 Axis vi. Moment of Interia about 3 Axis	Flexural Stiff.	Shear Stiff.	mbr. [] Layered	Clause 3.4
	Property/Stiffness Modifiers: a) Columns: i. Shear Area in 2 Direction ii. Shear Area in 3 Direction iii. Moment of Interia about 2 Axis vi. Moment of Interia about 3 Axis b) Main Beams & Secondary Beams:			mbr. [] Layered	Clause 3.4
	Property/Stiffness Modifiers: a) Columns: i. Shear Area in 2 Direction ii. Shear Area in 3 Direction iii. Moment of Interia about 2 Axis vi. Moment of Interia about 3 Axis b) Main Beams & Secondary Beams: i. Shear Area in 2 Direction	Flexural Stiff.	Shear Stiff.	mbr. [] Layered	Clause 3.4 Table 3-1
	Property/Stiffness Modifiers: a) Columns: i. Shear Area in 2 Direction ii. Shear Area in 3 Direction iii. Moment of Interia about 2 Axis vi. Moment of Interia about 3 Axis b) Main Beams & Secondary Beams: i. Shear Area in 2 Direction ii. Shear Area in 3 Direction	Flexural Stiff.	Shear Stiff.	mbr. [] Layered	Clause 3.4 Table 3-1 Clause 3.4
	Property/Stiffness Modifiers: a) Columns: i. Shear Area in 2 Direction ii. Shear Area in 3 Direction iii. Moment of Interia about 2 Axis vi. Moment of Interia about 3 Axis b) Main Beams & Secondary Beams: i. Shear Area in 2 Direction ii. Shear Area in 3 Direction ii. Shear Area in 3 Direction iii. Torsional Costant	Flexural Stiff.	Shear Stiff.	mbr. [] Layered	Clause 3.4 Table 3-1
	Property/Stiffness Modifiers: a) Columns: i. Shear Area in 2 Direction ii. Shear Area in 3 Direction iii. Moment of Interia about 2 Axis vi. Moment of Interia about 3 Axis b) Main Beams & Secondary Beams: i. Shear Area in 2 Direction iii. Shear Area in 3 Direction iii. Shear Area in 3 Direction iii. Torsional Costant iv. Moment of Interia about 2 Axis	Flexural Stiff.	Shear Stiff.	mbr. [] Layered	Clause 3.4 Table 3-1 Clause 3.4
	Property/Stiffness Modifiers: a) Columns: i. Shear Area in 2 Direction ii. Shear Area in 3 Direction iii. Moment of Interia about 2 Axis vi. Moment of Interia about 3 Axis b) Main Beams & Secondary Beams: i. Shear Area in 2 Direction ii. Shear Area in 3 Direction ii. Shear Area in 3 Direction iii. Torsional Costant	Flexural Stiff.	Shear Stiff.	mbr. [] Layered	Clause 3.4 Table 3-1 Clause 3.4
	Property/Stiffness Modifiers: a) Columns: i. Shear Area in 2 Direction ii. Shear Area in 3 Direction iii. Moment of Interia about 2 Axis vi. Moment of Interia about 3 Axis b) Main Beams & Secondary Beams: i. Shear Area in 2 Direction iii. Shear Area in 3 Direction iii. Shear Area in 3 Direction iii. Torsional Costant iv. Moment of Interia about 2 Axis	Flexural Stiff.	Shear Stiff.	mbr. [] Layered	Clause 3.4 Table 3-1 Clause 3.4
	Property/Stiffness Modifiers: a) Columns: i. Shear Area in 2 Direction ii. Shear Area in 3 Direction iii. Moment of Interia about 2 Axis vi. Moment of Interia about 3 Axis b) Main Beams & Secondary Beams: i. Shear Area in 2 Direction iii. Torsional Costant iv. Moment of Interia about 2 Axis v. Moment of Interia about 2 Axis	Flexural Stiff.	Shear Stiff.	mbr. [] Layered	Clause 3.4 Table 3-1 Clause 3.4
	Property/Stiffness Modifiers: a) Columns: i. Shear Area in 2 Direction ii. Shear Area in 3 Direction iii. Moment of Interia about 2 Axis vi. Moment of Interia about 3 Axis b) Main Beams & Secondary Beams: i. Shear Area in 2 Direction iii. Shear Area in 3 Direction iii. Shear Area in 3 Direction iii. Torsional Costant iv. Moment of Interia about 2 Axis c) Shear Wall:	Flexural Stiff.	Shear Stiff.	mbr. [] Layered	Clause 3.4 Table 3-1 Clause 3.4
	Property/Stiffness Modifiers: a) Columns: i. Shear Area in 2 Direction ii. Shear Area in 3 Direction iii. Moment of Interia about 2 Axis vi. Moment of Interia about 3 Axis b) Main Beams & Secondary Beams: i. Shear Area in 2 Direction iii. Shear Area in 3 Direction iii. Shear Area in 3 Direction iii. Shear Area in 3 Direction iii. Torsional Costant iv. Moment of Interia about 2 Axis v. Moment of Interia about 3 Axis	Flexural Stiff.	Shear Stiff.	mbr. [] Layered	Clause 3.4 Table 3-1 Clause 3.4
	Property/Stiffness Modifiers: a) Columns: i. Shear Area in 2 Direction ii. Shear Area in 3 Direction iii. Moment of Interia about 2 Axis vi. Moment of Interia about 3 Axis b) Main Beams & Secondary Beams: i. Shear Area in 2 Direction iii. Torsional Costant iv. Moment of Interia about 2 Axis c) Shear Wall: Stiffness of Cracked or Un-Cracked i. Membrane F11 Direction	Flexural Stiff.	Shear Stiff.	mbr. [] Layered	Clause 3.4 Table 3-1 Clause 3.4 Table 3-1
	Property/Stiffness Modifiers: a) Columns: i. Shear Area in 2 Direction ii. Shear Area in 3 Direction iii. Moment of Interia about 2 Axis vi. Moment of Interia about 3 Axis b) Main Beams & Secondary Beams: i. Shear Area in 2 Direction iii. Torsional Costant iv. Moment of Interia about 2 Axis v. Moment of Interia about 3 Axis c) Shear Wall: Stiffness of Cracked or Un-Cracked i. Membrane F11 Direction ii. Membrane F12 Direction	Flexural Stiff.	Shear Stiff.	mbr. [] Layered	Clause 3.4 Table 3-1 Clause 3.4
	Property/Stiffness Modifiers: a) Columns: i. Shear Area in 2 Direction ii. Shear Area in 3 Direction iii. Moment of Interia about 2 Axis vi. Moment of Interia about 3 Axis b) Main Beams & Secondary Beams: i. Shear Area in 2 Direction iii. Shear Area in 2 Direction iii. Shear Area in 3 Direction iii. Torsional Costant iv. Moment of Interia about 2 Axis v. Moment of Interia about 2 Axis v. Moment of Interia about 3 Axis c) Shear Wall: Stiffness of Cracked or Un-Cracked i. Membrane F11 Direction ii. Membrane F12 Direction iii. Membrane F12 Direction iv. Bending M11 Direction	Flexural Stiff.	Shear Stiff.	mbr. [] Layered	Clause 3.4 Table 3-1 Clause 3.4 Table 3-1 Clause 3.4
	Property/Stiffness Modifiers: a) Columns: i. Shear Area in 2 Direction ii. Shear Area in 3 Direction iii. Moment of Interia about 2 Axis vi. Moment of Interia about 3 Axis b) Main Beams & Secondary Beams: i. Shear Area in 2 Direction iii. Shear Area in 2 Direction iii. Shear Area in 3 Direction iii. Torsional Costant iv. Moment of Interia about 2 Axis v. Moment of Interia about 2 Axis v. Moment of Interia about 2 Axis v. Moment of Interia about 3 Axis c) Shear Wall: Stiffness of Cracked or Un-Cracked i. Membrane F11 Direction ii. Membrane F12 Direction iii. Membrane K12 Direction v. Bending M11 Direction v. Bending M22 Direction	Flexural Stiff.	Shear Stiff.	mbr. [] Layered	Clause 3.4 Table 3-1 Clause 3.4 Table 3-1 Clause 3.4
	Property/Stiffness Modifiers: a) Columns: i. Shear Area in 2 Direction ii. Shear Area in 3 Direction iii. Moment of Interia about 2 Axis vi. Moment of Interia about 3 Axis b) Main Beams & Secondary Beams: i. Shear Area in 2 Direction iii. Shear Area in 2 Direction iii. Shear Area in 3 Direction iii. Torsional Costant iv. Moment of Interia about 2 Axis v. Moment of Interia about 3 Axis c) Shear Wall: Stiffness of Cracked or Un-Cracked i. Membrane F11 Direction ii. Membrane F12 Direction iii. Membrane F12 Direction v. Bending M11 Direction v. Bending M12 Direction	Flexural Stiff.	Shear Stiff.	mbr. [] Layered	Clause 3.4 Table 3-1 Clause 3.4 Table 3-1 Clause 3.4
	Property/Stiffness Modifiers: a) Columns: i. Shear Area in 2 Direction ii. Shear Area in 3 Direction iii. Moment of Interia about 2 Axis vi. Moment of Interia about 3 Axis b) Main Beams & Secondary Beams: i. Shear Area in 2 Direction iii. Shear Area in 2 Direction iii. Shear Area in 3 Direction iii. Torsional Costant iv. Moment of Interia about 2 Axis v. Moment of Interia about 2 Axis v. Moment of Interia about 2 Axis v. Moment of Interia about 3 Axis c) Shear Wall: Stiffness of Cracked or Un-Cracked i. Membrane F11 Direction ii. Membrane F12 Direction iii. Membrane K12 Direction v. Bending M11 Direction v. Bending M22 Direction	Flexural Stiff.	Shear Stiff.	mbr. [] Layered	Clause 3.4 Table 3-1 Clause 3.4 Table 3-1 Clause 3.4
	Property/Stiffness Modifiers: a) Columns: i. Shear Area in 2 Direction ii. Shear Area in 3 Direction iii. Moment of Interia about 2 Axis vi. Moment of Interia about 2 Axis b) Main Beams & Secondary Beams: i. Shear Area in 2 Direction iii. Shear Area in 2 Direction iii. Shear Area in 3 Direction iii. Shear Area in 3 Direction iii. Torsional Costant iv. Moment of Interia about 2 Axis v. Moment of Interia about 2 Axis v. Moment of Interia about 3 Axis c) Shear Wall: Stiffness of Cracked or Un-Cracked i. Membrane F11 Direction iii. Membrane F12 Direction iv. Bending M11 Direction v. Bending M12 Direction vi. Bending M12 Direction vi. Shear V13 Direction	Flexural Stiff. Flexural Stiff. Fl	Shear Stiff. Shear Stiff. Shear Stiff. Un-Cracked		Clause 3.4 Table 3-1 Clause 3.4 Table 3-1 Clause 3.4 Table 3-1
	Property/Stiffness Modifiers: a) Columns: i. Shear Area in 2 Direction iii. Shear Area in 3 Direction iii. Moment of Interia about 2 Axis vi. Moment of Interia about 3 Axis b) Main Beams & Secondary Beams: i. Shear Area in 2 Direction iii. Shear Area in 3 Direction iii. Shear Area in 3 Direction iii. Torsional Costant iv. Moment of Interia about 2 Axis v. Moment of Interia about 2 Axis v. Moment of Interia about 2 Axis v. Moment of Interia about 3 Axis c) Shear Wall: Stiffness of Cracked or Un-Cracked i. Membrane F11 Direction iii. Membrane F22 Direction iii. Membrane F12 Direction iv. Bending M11 Direction v. Bending M12 Direction vi. Bending M12 Direction vii. Shear V13 Direction vii. Shear V23 Direction viii. Shear V23 Direction	Flexural Stiff. Flexural Stiff. Fl	Shear Stiff. Shear Stiff. Shear Stiff. Un-Cracked		Clause 3.4 Table 3-1 Clause 3.4 Table 3-1 Clause 3.4 Table 3-1
	Property/Stiffness Modifiers: a) Columns: i. Shear Area in 2 Direction iii. Moment of Interia about 2 Axis vi. Moment of Interia about 2 Axis b) Main Beams & Secondary Beams: i. Shear Area in 2 Direction iii. Torsional Costant iv. Moment of Interia about 2 Axis v. Moment of Interia about 2 Axis c) Shear Area in 3 Direction iii. Torsional Costant iv. Moment of Interia about 2 Axis v. Moment of Interia about 2 Axis c) Shear Wall: Stiffness of Cracked or Un-Cracked i. Membrane F11 Direction iii. Membrane F22 Direction iii. Membrane F12 Direction v. Bending M11 Direction v. Bending M12 Direction vii. Shear V13 Direction vii. Shear V23 Direction viii. Shear V23 Direction viii. Shear V23 Direction viii. Shear V23 Direction	Flexural Stiff. Flexu	Shear Stiff. Shear Stiff. Shear Stiff. Un-Cracked ers are expressed in a		Clause 3.4 Table 3-1 Clause 3.4 Table 3-1 Clause 3.4 Table 3-1
	Property/Stiffness Modifiers: a) Columns: i. Shear Area in 2 Direction iii. Shear Area in 3 Direction iii. Moment of Interia about 2 Axis vi. Moment of Interia about 3 Axis b) Main Beams & Secondary Beams: i. Shear Area in 2 Direction iii. Shear Area in 3 Direction iii. Shear Area in 3 Direction iii. Torsional Costant iv. Moment of Interia about 2 Axis v. Moment of Interia about 2 Axis v. Moment of Interia about 2 Axis v. Moment of Interia about 3 Axis c) Shear Wall: Stiffness of Cracked or Un-Cracked i. Membrane F11 Direction iii. Membrane F22 Direction iii. Membrane F12 Direction iv. Bending M11 Direction v. Bending M12 Direction vi. Bending M12 Direction vii. Shear V13 Direction vii. Shear V23 Direction viii. Shear V23 Direction	Flexural Stiff. Flexural Stiff. Flexural Stiff. Image: Stiff.	Shear Stiff. Shear Stiff. Shear Stiff. Un-Cracked		Clause 3.4 Table 3-1 Clause 3.4 Table 3-1 Clause 3.4 Table 3-1

D.7	Modal Combination Method:	[]CQC []SRSS			Clause 7.4	
D.8	Support Condition of the Foundation:	[] Fix	ed [] Hinge [] Roller	From Software	
Е	Result for Analysis of Building					
E.1	Auto Seismic User Coefficient	Weight	Base Shear			
	i. EqX-ULS			kN		
	ii. EqY-ULS			kN	From Software	
	iii. EqX-SLS			kN		
	iv. EqY-SLS			kN		
E.2	Base Reaction					
	a) For Ultimate Limit State:	FX (kN	FY			
	i. EqX-ULS (Linear Static)			kN		
	ii. EqY-ULS (Linear Static)			kN	From Software	
	iii. RsX-ULS(Linear Response Spectrum)			kN	1101102030,000	
	iv. RsY-ULS (Linear Response Spectrum)			kN	-	
		-	· · · · · · · · · · · · · · · · · · ·		1	
	b) For Serviceability Limit State:	FX	FY			
	i. EqX-SLS (Linear Static)			kN	4	
	ii. EqY-SLS (Linear Static)			kN	From Software	
	iii. RsX-SLS (Linear Response Spectrum)			kN		
	iv. RsY-SLS (Linear Response Spectrum)			kN		
E.3	Final Scale Factor (When VR <v) (after="" scaling):<="" td=""><td></td><td></td><td></td><td></td></v)>					
-	a) For Ultimate Limit State:					
	i. Factor in X - Direction= $(V_{ULS}/V_R)^*SF_{initial, ULS}$			kN	1	
	ii. Factor in Y - Direction= $(V_{ULS}/V_R)^*SF_{initial, ULS}$			kN	Clause 7.5	
	b) For Serviceability Limit State:					
	i. Factor in X - Direction= $(V_{SLS}/V_{R,SLS})$ *SF _{initial, SLS}			kN	-	
	ii. Factor in Y - Direction= $(V_{SLS}/V_{R,SLS})$ *SF _{initial, SLS}			kN		
E.4	Mass Participation Ratio	Acceleration				
2.1	a) No. of Modes Considered		<i>c</i> (
	Modal	Sum Ux	Sum Uy			
	i. 1st Modes Considered			mm/sec2		
	ii. 2nd Modes Considered			mm/sec2	Clause 7.3	
	iii. 3rd Modes Considered			mm/sec2		
	iv. Last Modes Considered for at least 90%			mm/sec2		
	b) The Modes with Natural Frequency less than 33 Hz			Hz		
E.5	Story Displacement of Building					
L.J	a) For Ultimate Limit State: (ULS: 0.025 * H) Permissible	Eq-X / Rs-X	Eq-Y / Rs-Y		-	
	15th Floor			mm		
	14th Floor			mm]	
	13th Floor			mm		
	12th Floor			mm		
	11th Floor			mm	1	
	10th Floor			mm	1	
	9th Floor			mm	Clause 5.6.1	
	8th Floor			mm	1	
	7th Floor			mm	-	
	6th Floor			mm		
	5th Floor					
				mm	1	
	4th Floor				4	
	3rd Floor			mm	4	
	2nd Floor 1st Floor			mm mm	1	
			· · ·		I	
	b) For Serviceabilty Limit State: (SLS: 0.006 * H) Permissible	Eq-X / Rs-X	Eq-Y / Rs-Y			
	15th Floor					

	01111001	I		N11	
	8th Floor			kN	Cuust 5.5.1.1
	9th Floor			kN	Clause 5.5.1.1
	11th Floor 10th Floor			kN kN	
	12th Floor 11th Floor			kN kN	
	13th Floor			kN	
	14th Floor			kN	
	15th Floor			kN I-N	
	164 19	Lateral	Strength	I NT	
E.7	Weak Story for Ultimate Limit State (ULS)		EL(i+1) Strongth		
		- 000/	EI (11)		
	1st Floor				
	2nd Floor				
	3rd Floor				
	4th Floor				
	5th Floor				
	6th Floor				
	7th Floor				
	8th Floor				Clause 5.6.3
	9th Floor				
	10th Floor				
	11th Floor				
	12th Floor				
	13th Floor				
	14th Floor				
	15th Floor				
	b) For Serviceabilty Limit State: (SLS: 0.006 Permissible)	Eq-X / Rs-X	Eq-Y / Rs-Y		
	1 st Floor				
	2nd Floor				
	3rd Floor				
	4th Floor				
	5th Floor				
	6th Floor				
	7th Floor				
	8th Floor				
	9th Floor				Clause 5.6.3
	10th Floor				
	11th Floor				
	12th Floor				
	13th Floor				
	14th Floor				
	15th Floor				
	a) For Ultimate Limit State: (ULS: 0.025 Permissible)	Eq-X / Rs-X	Eq-Y / Rs-Y		
E.6	Inter Story Deflection (Drifts)				
	15611001			mm	
-	1st Floor			mm	
	2nd Floor			mm mm	
	3rd Floor			mm	
	4th Floor			mm	
	5th Floor			mm	
	7th Floor 6th Floor			mm	
	8th Floor 7th Floor			mm	
	9th Floor			mm	Clause 5.6.1
	10th Floor			mm	
	11th Floor			mm	
	12th Floor			mm	

	74L El		Г	I NT	Г
	7th Floor 6th Floor			kN	-
	5th Floor			kN	-
	Sth Floor 4th Floor			kN	-
	3rd Floor			kN	-
	2nd Floor			kN LN	-
	1 st Floor			kN kN	-
E.8	Stown: Stiffnagg (Saft Stown)	Eq-X / Rs-X			
E.0	Storey Stiffness (Soft Story)	Eq-A / RS-A 70% of (i±1)	Eq-Y / Rs-Y 80% of (i±1)		-
	15th Floor			kN/m	-
	14th Floor			kN/m	-
	13th Floor			kN/m	-
	12th Floor			kN/m	1
	11th Floor			kN/m	-
	10th Floor			kN/m	-
	9th Floor			kN/m	Clause 5.5.1.2
	8th Floor			kN/m	
	7th Floor			kN/m	
	6th Floor			kN/m	1
	5th Floor			kN/m	
	4th Floor			kN/m	1
	3rd Floor			kN/m	1
	2nd Floor			kN/m]
	1st Floor			kN/m	
E.9	Mass Irregularity (Kg)	Kg	50% EL(i±1)		
	15th Floor				-
	14th Floor				1
	13th Floor]
	12th Floor]
	11th Floor]
	10th Floor]
	9th Floor				<i>Clause 5.5.1.5</i>
	8th Floor				Cuuse 5.5.1.5
	7th Floor			kg	
	6th Floor			kg	
	5th Floor			kg	
	4th Floor			kg	
	3rd Floor			kg	
	2nd Floor			kg	
	1st Floor			kg	
E.10	Torsion Irregularity for Ultimate Limit State (ULS):	Eq-X / Rs-X	Eq-Y / Rs-Y		
	Max Corner Displacement			mm	<i>Clause 5.5.2.1</i>
	Min Corner Displacement			mm	Clause 5.5.2.1
	Ratio				
E.11	Check for Re-entrant Corner Irregularity	[]Yes	[] No		Clause 5.5.2.2
E.12	Check for Diaphragm Discontinuity Irregularity	[] Yes	[] No		Clause 5.5.2.3
E.13	Check for Out-of-Plane Offset Irregularity	[] Yes	[] No		Clause 5.5.2.4
E.14	Dual System Check	X-Direction	Y-Direction		
	Percentage of Total Design Base Shear by Columns in X and Y Direction			%	- Clause 3.1
E.15	Sanaratian hatwaan Blacks (if annliashla)	[]Yes	[]No		
L.13	Separation between Blocks (if applicable) If Yes	Block A	Block B		1
	15th Floor	_ 10 000 / 1		mm	1
	14th Floor			mm	1
	13th Floor			mm	1

11th Floor			mm	
10th Floor			mm	(1)
9th Floor			mm	Clause 5.6.2
8th Floor			mm	
7th Floor			mm	
6th Floor			mm	
5th Floor			mm	
			mm	
			mm	
			mm	
1st Floor			mm	
Coological Investigation and Design of Foundations				
	[]] Vas	[]No	1	
-		See Delow		
_				
	F 137	F 131	Г	
	[]Yes	[] No		
			kN/m ²	Soil Report
				-
				Clause 4.1.3
Water Table:				Soil Report
1				-
	[] Yes	[] No		NBC 205 T:3-1
Site Considertion as per NBC 108 ?	[] Yes	[] No		NBC 108:1994
If No				
Terrain (Slope or Flat)				NBC 108:1994
Software used for design of foundation along with version :				Version
Foundation System	[] Mat [] Con	nbined [] Isolated	1 [] Strap [] Pile	
Calculated Maximum Pressure on Foundation:			kN/m ²	
Thickness of Foundation			mm	From Software
Punching Shear Ratio (Less than 1)				
Design of Structural Flomenta of Concrete				
5				
	[] IS 456-20	00		
)	
	[] NBC 103			Defenence Code
iii. Design of Structural Elements Foundation, Slab, Staircase, etc: iv. Others	[] 15 450:20)	Reference Code
IV. Others		00)	Reference Code
		00)	Reference Code
Member Design		00)	Reference Code
Member Design a) Check for All Members Passed or Failed	[]Yes	00 []No	, 	Reference Code
a) Check for All Members Passed or Failed		[]No		
a) Check for All Members Passed or Failed If No	Design/Ch	[] No eck Again		Reference Code
a) Check for All Members Passed or Failed If No i. Design of Columns	Design/Ch	[] No eck Again [] No		
a) Check for All Members Passed or Failed If No i. Design of Columns ii. Design of Beams	Design/Ch []Yes []Yes	[] No eck Again [] No [] No		From Software
a) Check for All Members Passed or Failed If No i. Design of Columns ii. Design of Beams iii. Design of Shear Wall (if applicable)	Design/Ch [] Yes [] Yes [] Yes [] Yes	[] No eck Again [] No [] No [] No		
a) Check for All Members Passed or Failed If No i. Design of Columns ii. Design of Beams iii. Design of Shear Wall (if applicable) Design of Tie/Plinth Beam	Design/Ch [] Yes [] Yes [] Yes [] Yes [] Yes [] Yes	[] No eck Again [] No [] No [] No [] No [] No		From Software
a) Check for All Members Passed or Failed If No i. Design of Columns ii. Design of Beams iii. Design of Shear Wall (if applicable) Design of Tie/Plinth Beam Design of Critical Panel of Slabs	Design/Ch [] Yes	[] No eck Again [] No [] No [] No [] No [] No [] No		From Software
a) Check for All Members Passed or Failed If No i. Design of Columns ii. Design of Beams iii. Design of Shear Wall (if applicable) Design of Tie/Plinth Beam Design of Critical Panel of Slabs Design of Cantilever Slabs (Without Beam Supports)	Design/Ch []Yes	[] No eck Again [] No [] No [] No [] No [] No [] No [] No		From Software
a) Check for All Members Passed or Failed If No i. Design of Columns ii. Design of Beams iii. Design of Shear Wall (if applicable) Design of Tie/Plinth Beam Design of Critical Panel of Slabs Design of Cantilever Slabs (Without Beam Supports) Design of Waffle Slabs	Design/Ch []Yes	[] No eck Again [] No [] No		From Software
a) Check for All Members Passed or Failed If No i. Design of Columns ii. Design of Beams iii. Design of Shear Wall (if applicable) Design of Tie/Plinth Beam Design of Critical Panel of Slabs Design of Cantilever Slabs (Without Beam Supports) Design of Waffle Slabs Design of Deep Beams	Design/Ch []Yes	[] No eck Again [] No [] No		From Software Clause 5.2/5.3 (A)
a) Check for All Members Passed or Failed If No i. Design of Columns ii. Design of Beams iii. Design of Shear Wall (if applicable) Design of Tie/Plinth Beam Design of Critical Panel of Slabs Design of Cantilever Slabs (Without Beam Supports) Design of Deep Beams Design of Corbel	Design/Ch []Yes	[] No eck Again [] No [] No		From Software
a) Check for All Members Passed or Failed If No i. Design of Columns ii. Design of Beams iii. Design of Shear Wall (if applicable) Design of Tie/Plinth Beam Design of Critical Panel of Slabs Design of Cantilever Slabs (Without Beam Supports) Design of Deep Beams Design of Corbel Design of Cantilever Beams	Design/Ch []Yes	[] No eck Again [] No [] No		From Software Clause 5.2/5.3 (A)
a) Check for All Members Passed or Failed If No i. Design of Columns ii. Design of Beams iii. Design of Shear Wall (if applicable) Design of Tie/Plinth Beam Design of Critical Panel of Slabs Design of Cantilever Slabs (Without Beam Supports) Design of Deep Beams Design of Corbel Design of Cantilever Beams Design of Staircases (Dog Legged, Open-well & Helical)	Design/Ch []Yes []Yes	[] No eck Again [] No [] No		From Software Clause 5.2/5.3 (A)
a) Check for All Members Passed or Failed If No i. Design of Columns ii. Design of Beams iii. Design of Shear Wall (if applicable) Design of Tie/Plinth Beam Design of Critical Panel of Slabs Design of Cantilever Slabs (Without Beam Supports) Design of Deep Beams Design of Corbel Design of Cantilever Beams Design of Staircases (Dog Legged, Open-well & Helical) Design of Foundations (Isolated, Combined, Strap Footing)	Design/Ch []Yes	[] No eck Again [] No [] No		From Software Clause 5.2/5.3 (A)
a) Check for All Members Passed or Failed If No i. Design of Columns ii. Design of Beams iii. Design of Shear Wall (if applicable) Design of Tie/Plinth Beam Design of Critical Panel of Slabs Design of Cantilever Slabs (Without Beam Supports) Design of Deep Beams Design of Deep Beams Design of Corbel Design of Cantilever Beams Design of Staircases (Dog Legged, Open-well & Helical) Design of Foundations (Isolated, Combined, Strap Footing)	Design/Ch []Yes	[] No eck Again [] No [] No		From Software Clause 5.2/5.3 (A)
a) Check for All Members Passed or Failed If No i. Design of Columns ii. Design of Beams iii. Design of Shear Wall (if applicable) Design of Tie/Plinth Beam Design of Critical Panel of Slabs Design of Cantilever Slabs (Without Beam Supports) Design of Waffle Slabs Design of Deep Beams Design of Corbel Design of Cantilever Beams Design of Staircases (Dog Legged, Open-well & Helical) Design of Foundations (Isolated, Combined, Strap Footing) Design of Retaining Wall	Design/Ch []Yes	[] No eck Again [] No [] No		From Software Clause 5.2/5.3 (A)
a) Check for All Members Passed or Failed If No i. Design of Columns ii. Design of Beams iii. Design of Shear Wall (if applicable) Design of Tie/Plinth Beam Design of Critical Panel of Slabs Design of Cantilever Slabs (Without Beam Supports) Design of Deep Beams Design of Deep Beams Design of Corbel Design of Cantilever Beams Design of Staircases (Dog Legged, Open-well & Helical) Design of Foundations (Isolated, Combined, Strap Footing)	Design/Ch []Yes	[] No eck Again [] No [] No		From Software Clause 5.2/5.3 (A)
	iquefaction Potential: dopted Value as per NBC 205, Table 3-1 ite Considertion as per NBC 108 ? If No Ferrain (Slope or Flat) oftware used for design of foundation along with version : oundation System Calculated Maximum Pressure on Foundation: hickness of Foundation	3rd Floor 2nd Floor 1st Floor Ist Floor Geological Investigation and Design of Foundations ieological Investigation Conducted [] Yes Geological Investigation Conducted [] Yes Iame of the Consulting Firm: [] Yes Iame of Designer: [] Yes IEC Council No.: [] Yes Designer Master Degree: []] Yes Illowable Bearing Capacity of Soil [] Yes I-value: [] Yes ype of Soil: [] Yes vater Table: [] Yes idupted Value as per NBC 205, Table 3-1 []] Yes it Considertion as per NBC 108 ? [] Yes If No [] Yes oftware used for design of foundation along with version : [] Mat [] Con oundation System []] Mat [] Con alculated Maximum Pressure on Foundation: [] Mat [] Con hickness of Foundation [] Imathickness of Foundation unching Shear Ratio (Less than 1) [] Is 456:20 ii. Concrete Design Code: [] IS 456:20 ii. Design & Detailing of Reinforced Concrete Structures: [] NBC 105:	3rd Floor	3rd Floor mm 2nd Floor mm 1st Floor mm 1st Floor mm Geological Investigation and Design of Foundations mm ieological Investigation Conducted []Yes]No Idress See Below Iame of the Consulting Firm: Image: Conscience of the Consulting Firm: Iame of Designer: Image: Conscience of the Conscience of Concrete Design Code Referred: [] Is 456:2000

	If No	Provide 3 Cri	tical Samples		Manual Calcuation
G.16	Check for Beam-Column Joints	[]Yes	[] No		Clause 4.4
G.17	Column Size on Basis of Horizontal Development Length			mm	Clause 4.4.2
G.18	Minimum Diameter of Transverse Rebar in Beam/Column on Basis of Lapping/Splicing Zone			mm	Clause 4.5.1 (g)
G.19	Minimum Diameter of Special Confining Column Ties For Rectangular/Square/Circular Column			mm	Clause 4.3- (4.3.2/4.33)
	Design of Other Members:				
G.20	Detailed Design of other Structural Members such as Roof Truss, Steel Structural Members etc (if any)	[] Yes	[] No		
~ • •					
G.22	Check for Max. & Min. Percentage of Reinforcement				
	i. Max. Percentage of Rebars provided in Columns			%	Clause 4.2.2
	ii. Max. Percentage of Rebars provided in Beams			%	Ciause 4.2.2
	iii. Max. Percentage of Rebars provided in Shear Walls			%	

I/We hereby certify that the proposed design of building (.....) and its various components comply all the requirements of Nepal National Building Code. I/We also affirm that the submitted checklist is done by the concerned Engineers and Architects duly registered in Nepal Engineering Council. The data made available in this checklist have been filled following the respective guidelines.

Owner's (Client) Name: Signature: **Designer's Name:** Signature:

Stamp

Stamp

ump

Design Checked By: